ENGINEERING

# On the interdependence between the direction of a force and certain mechanical functions, such as reactions, axial forces, bending moments, etc. 

DIE BEZIEHUNG ZWISCHEN EINER KRAFTRICHTUNG UND GEWISSEN MECHANISCHEN FUNKTIONEN, WIE AUFLAGERKRÄFTE, AXIALKRÄFTE, BIEGUNGSMOMENTE USW.

LA RELATION ENTRE LA DIRECTION D'UNE FORCE ET CERTAINES FONCTIONS MÉCANIQUES, TELLE QUE LES RÉACTIONS, LES EFFORTS LONGITUDINAUX, LES MOMENTS FLÉCHISSANTS, ETC.

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#### Abstract

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# ON THE INTERDEPENDENCE BETWEEN THE DIRECTION OF A FORCE AND CERTAIN MECHANICAL FUNCTIONS, SUCH AS REACTIONS, AXIAL FORCES, BENDING MOMENTS, ETC. 

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LA RELATION ENTRE LA DIRECTION D'UNE FORCE ET CERTAINES<br>FONCTIONS MÉCANIQUES, TELLE QUE LES RÉACTIONS, LES EFFORTS LONGITUDINAUX, LES MOMENTS FLÉCHISSANTS, ETC.

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A concentrated force $P$ is applied at a certain point $F$ of an elastic system and is acting at an angle $\alpha$ to the horizontal. It causes certain deformations and tensions in different points of the system. Certain axial forces, transverse forces and bending moments arise in imaginary sections. Reaction is exercised on the system at its bearings or joints.

All the above-mentioned values are functions of the force $P$ and of the angle $\alpha$. Some of them (e.g. reactions of fixed bearings) are vectors, i. e. they are determined by tension and direction. Others are determined by tension and sign (e.g. components of reaction in a certain direction, displacements in certain directions, axial forces in rods, transverse forces in certain sections, bending moments about a certain axis).

Such values with a variable direction may be resolved into two components having two freely established directions and their study may therefore be limited to the study of functions with a constant direction.

Such a value will be called effect. There are a great many of such effects, for instance $S$. For a given $P, S$ ist a function of $\alpha$ :

$$
S=f(\alpha)
$$

Let us take two directions of action of the force, at right angles to one another, viz. the vertical direction, $\alpha=90^{\circ}$, downwards, and the horizontal direction from right to left, $\alpha=0$, and resolve the force $P$ into two components (Fig. 1).

$$
\begin{align*}
& H=P \cos \alpha  \tag{1}\\
& V=P \sin \alpha \tag{2}
\end{align*}
$$

If only the horizontal component is acting, the result will be

$$
\begin{equation*}
S_{h}=x \cdot H \tag{3}
\end{equation*}
$$

If only the vertical component is acting, it will be

$$
\begin{equation*}
S_{v}=\lambda \cdot V \tag{4}
\end{equation*}
$$

presuming the proportionality of cause and effect i. e. of the law of superposition. $x$ and $\lambda$ are coefficients of proportionality. If the effect $S$ has the same dimension as the force, then $x$ and $\lambda$ have no dimension at all. Their dimension may be obtained in general, from equations (3) and (4).

In view of the fact that the forces $H$ and $V$ are acting simultaneously, we have according to the law of superposition

$$
\begin{equation*}
S=S_{h}+S_{v} \tag{5}
\end{equation*}
$$

Introducing equations (3) and (4) into (5), the result is as follows: (taking into consideration the formulae (1) and (2))

$$
\begin{equation*}
S=P(\% \cos \alpha+\lambda \sin \alpha) \tag{6}
\end{equation*}
$$



Fig. 1.
Fig. 2.
Let us say:

$$
\begin{align*}
\frac{\%}{\lambda} & =\tan \varphi=t  \tag{7}\\
\text { then } \sec \varphi & =\sqrt{1+t^{2}}  \tag{8}\\
\text { or } \sec \varphi & =\frac{1}{\lambda} \sqrt{\varkappa^{2}+\iota^{2}}  \tag{9}\\
S & =P \lambda \varrho \tag{10}
\end{align*}
$$

besides

$$
\varrho=\sin \alpha+\tan \varphi \cos \alpha=\sec \varphi(\sin \alpha \cos \varphi+\cos \alpha \sin \varphi)=\sec \varphi \sin (\alpha+\varphi)
$$

Introducing this formula into (10), and taking into consideration the formula (9),

$$
\begin{align*}
& D=P \sqrt{\varkappa^{2}+\lambda^{2}}  \tag{11}\\
& S=D \sin (\varphi+\alpha) \tag{12}
\end{align*}
$$

$S$ changes dependently on $\alpha$, along the sinusoid.
If $\beta=\alpha+\varphi$

$$
\begin{equation*}
\text { then } S=D \sin \tag{13}
\end{equation*}
$$

The axis $O Y$ is drawn downwards, at the angle $\varphi$ to the horizontal line and $O A=S$ is drawn, at the angle $\beta$ to the axis $O Y$ (Fig. 2), $S$ and $\beta$ are then polar coordinates of the point A.

Thence:

$$
\begin{align*}
& A Y=y=S \sin \beta  \tag{14}\\
& A X=x=S \cos \beta \tag{15}
\end{align*}
$$

If $O O^{\prime}$ is at right angles to $O Y$ and $O O^{\prime}=\frac{1}{2} D$
and if then

$$
X O^{\prime}=y^{\prime} \quad \text { and } \quad x^{\prime}=x
$$

$$
y^{\prime}=y-\frac{1}{2} D
$$

and

$$
\begin{equation*}
x^{\prime 2}+y^{\prime 2}=x^{2}+y^{2}-y D+\left(\frac{D}{2}\right)^{2} \tag{16}
\end{equation*}
$$



Fig. 3.


Fig. 4.

But according to equations (14) and (15)

$$
\begin{equation*}
y^{2}+x^{2}=S^{2} \tag{17}
\end{equation*}
$$

and according to equations (14) and (13)

$$
\begin{equation*}
y D=S^{2} \tag{18}
\end{equation*}
$$

Introducing (17) and (18) into (16), we obtain:

$$
x^{\prime 2}+y^{\prime 2}=\left(\frac{1}{2} D\right)^{2}
$$

The point $A$ lies therefore on the circumference of a circle, the radius of which is $\frac{D}{2}$ and the centre of which is at $O^{\prime}$, thus being tangential to $O Y$ at $O$. This anyway results directly from the geometrical solution. If

$$
T=D \cos \beta
$$

then $D$ is the hypotenuse of the rectangular triangle $A B C$ (Fig. 3), $D$ is therefore the diameter of a circle drawn about the triangle $A B C$. It follows that the apex $C^{\prime}$ of the right angle of the triangle $A C^{\prime} B$, lying on the same hypotenuse $A B$ and with its sides $A C^{\prime}=S$ and $B C^{\prime}=T$, lies on the same
circle. We displace the triangle $A C^{\prime} B$ until the diameter $A B$ is vertical; the side $A C^{\prime}$ will then become the radius which had to be obtained, and it appears that $C^{\prime}$ lies on the circumference of a circle of diameter $D$. The


Fig. 5.
relation between $S$ and $\beta$ in polar coordinates is determined by a circle tangential to the axis $O X$ (Fig. 2) inclined to the vertical line at the angle $\varphi$. If $\beta<0$, or $\beta>\pi$, then $S<0$. A second circle corresponds to the negative values of $S$; this circle is situated on the opposite side of the axis $O Y$, which is tangent to both circles (Fig. 5). Thence the following general law:


Fig. 6.
When in a given system the law of superposition can be applied, any mechanical function (force in rods, bending moment, transverse force, tension) characterized by tension and sign and caused by the force $P$, acting at a certain point at an angle $\alpha$ to the horizontal, can be expressed by polar coordinates $S=f(\alpha)$ in the form of two circles tangential to each another externally and with the same diameter $D$, whereby $D=S$ max.

## Examples:

1. Girder on two supports (Fig. 6a).

Reaction of the support $A=V \frac{b}{l}=\frac{a}{l} P \sin \alpha$.
For

$$
\alpha=\frac{\pi}{2}, \quad A \max =\frac{b}{l} P=D
$$

therefore $A=D \sin \alpha$ (Fig. bb).
The bending moment at the point of application of the force $P$ is $M=A a=D^{\prime} \sin \alpha$, if $D^{\prime}=D a$. Figure 6 b is to be reckoned here as well but on a different scale, in which $D==D^{\prime}$. The moment at a certain point of the girder (Fig. 6a) as well as the transverse force and the deflection are also expressed according to $\alpha$, on a corresponding scale fig. 6 b .


Fig. 7.


Fig. 8.
2. Three-hinged arch (Fig. 7).

We shall consider the bending moment in the section $M(x, y)$ as an effect of the force $P$, applied at a certain point of the right half of the arch. This moment depends upon the lefthand reaction $R$. Its vertical component is

$$
A=\varkappa^{\prime} H+\lambda^{\prime} V .
$$

The horizontal component

$$
B=\varkappa^{\prime \prime} H+\lambda^{\prime \prime} V,
$$

besides which,

$$
\begin{aligned}
& H=P \cos \alpha \\
& V=P \sin \alpha
\end{aligned}
$$

and $\varkappa^{\prime}, \varkappa^{\prime \prime}$ and $\lambda^{\prime}, \lambda^{\prime \prime}$ are corresponding coefficients of proportionality. According to Fig. 7:

$$
M=A x-B y .
$$

Introducing the angles $\varphi^{\prime}$ and $\varphi^{\prime \prime}$ as per equations:

$$
\tan \varphi^{\prime}=\frac{\varkappa^{\prime}}{\lambda^{\prime}} \quad \tan \varphi^{\prime \prime}=\frac{\varkappa^{\prime \prime}}{\lambda^{\prime \prime}}
$$

it follows that

$$
\begin{aligned}
A & =P\left(\varkappa^{\prime} \cos \alpha+\lambda^{\prime} \sin \alpha\right) \\
& =P \sqrt{\varkappa^{\prime 2}+\lambda^{\prime 2}} \cdot \sin \left(\alpha+\varphi^{\prime}\right)
\end{aligned}
$$

and

$$
B=P \sqrt{\varkappa^{\prime \prime 2}+\lambda^{\prime \prime 2}} \cdot \sin \left(\alpha+\varphi^{\prime \prime}\right)
$$

Therefore

$$
\begin{equation*}
M=P \cdot\left[x^{\prime} \sin \left(\alpha+\varphi^{\prime}\right)-y^{\prime} \sin \left(\alpha+\varphi^{\prime \prime}\right)\right] \tag{a}
\end{equation*}
$$

besides which,

$$
\begin{aligned}
& x^{\prime}=x \sqrt{\varkappa^{\prime 2}+\lambda^{\prime 2}} \\
& y^{\prime}=y \sqrt{\varkappa^{\prime \prime 2}+\lambda^{\prime \prime 2}}
\end{aligned}
$$

But

$$
\begin{aligned}
& \sin \left(\alpha+\varphi^{\prime}\right)=\sin \alpha \cos \varphi^{\prime}+\cos \alpha \sin \varphi^{\prime} \\
& \sin \left(\alpha+\varphi^{\prime \prime}\right)=\sin \alpha \cos \varphi^{\prime \prime}+\cos \alpha \sin \varphi^{\prime \prime}
\end{aligned}
$$

and as per (a)

$$
\frac{M}{P}=\varkappa^{\prime \prime \prime} \cos \alpha+\lambda^{\prime \prime \prime} \sin \alpha
$$

besides which,

$$
\begin{aligned}
& x^{\prime \prime \prime}=x^{\prime} \sin \varphi^{\prime}-y^{\prime} \sin \varphi^{\prime \prime} \\
& \lambda^{\prime \prime \prime}=x^{\prime} \cos \varphi^{\prime}-y^{\prime} \cos \varphi^{\prime \prime \prime}
\end{aligned}
$$

Finally

$$
\begin{equation*}
M=P \sqrt{\varkappa^{\prime \prime \prime} 2+\lambda^{\prime \prime \prime 2}} \cdot \sin \left(\alpha+\varphi^{\prime \prime \prime}\right) \tag{b}
\end{equation*}
$$

if

$$
\tan \varphi^{\prime \prime \prime}=\varkappa^{\prime \prime \prime}: \lambda^{\prime \prime \prime}
$$

We have then obtained here also the formula (b) in accordance with the equation (12).

## Conclusions:

1. Any mechanical function $M$ in a certain system has the property that for every point of application of the force there are two directions at right angles to each other, such that when the force is acting in one of these directions $M=0$, and when it is acting in the other $M=\max$.
2. If a certain mechanical function is equal to 0 for a certain angle $\alpha$, it is also equal to 0 in the opposite direction $\alpha= \pm 180^{\circ}$. When $\alpha= \pm 90^{\circ}$ it reaches its maximum.
3. For any point of the system and for a certain mechanical function of this system, there exists always one direction of the force for which this function equals 0 . The same applies to the directly opposite direction. For directions at right angles to each other, the function reaches its positive or negative maximum respectively. There cannot exist two directions not at right angles to each other, for which the mechanical function $M$ would equal 0 and the maximum respectively.
4. If a force of constant value and variable in direction is acting at a certain point of a girder or of a mechanical system (Fig. 8), it causes in all mechanical functions of this system, having a constant direction, alterations from 0 to $+\max$ and from $+\max$ to 0 , then from 0 to $-\max$ and from - max to 0 , with every complete rotation of the direction of the force.

## Examples of application.

Example 1. To determine $S$ max, when $P$ is acting at $C$ (Fig. 9a). When the force $P$ is acting on $C B$, then $S=0$. When the force $P$ is acting on $A C$, then $S=P$.
$C^{\prime} B^{\prime} \| C B$ and $C^{\prime} A^{\prime} \| C A$ are drawn from an arbitrarly chosen point $C$ (Fig. 9 b ). $C^{\prime} A^{\prime}=P$ and the circle $C^{\prime} A^{\prime} D^{\prime}$ tangential to $C^{\prime} B^{\prime}$ is drawn througli the points $C^{\prime}$ and $A^{\prime}$. The vertical line $C^{\prime} D^{\prime}$ at right angles to $C^{\prime} B^{\prime}$ gives $S$ max.


Fig. 9.
Example 2. To determine the internal stress in the diagonal $k$ of the lattice girder, when the force $P=1$ is applied at $C$ and has the direction shown on Fig. 10 a :


Fig. 10.
Given $l=24 \mathrm{~m}, h=3,6 \mathrm{~m}, a=l / 8=3 \mathrm{~m}$.
The length of the diagonal is $k=\sqrt{3^{2}+3,6^{2}}=\sqrt{9+12,95}=\sqrt{21,95}=4,69 \mathrm{~m}$.
When $\quad P_{v}=1$, then $A_{v}=\frac{3}{8}$.

$$
K_{v}=A_{v} \frac{k}{h}=\frac{3}{8} \cdot \frac{4,69}{3,6}=0,489 .
$$

From an arbitrarly chosen point $O$ (Fig. 10 b ) a vertical section $O V=K_{v}$ $=0,489$ and straight lines $O M \| C B$ and $O N$ at right angles to $C B$ are drawn.

The axis of symmetry of the section $O V$ cuts $O N$ at the centre of the circle $S$. The chord $O D \| P$ gives the internal stress $O D=K$ in the diagonal corresponding to the direction of the force $P$. At the same time it can be seen that $K=\max =O E$, when the force $P=1$ acts vertically to $C B$.

Example 3. A semicircular three-hinged arch of radius $r$. The force $P$ is applied at $D$ (Fig. 11), where the inclination of the normal is $45^{\circ}$ to the horizontal. To determine the maximum moment in $D$ and $E\left(\alpha=\frac{45}{2}\right)$, (Fig. 13) and the corresponding directions of the force $P=1$.


Fig. 11 und 12.


Fig. 13.

If the direction of the force is $D B$, the direction of the reaction at $B$ is $B D$ and $M_{D}=0$. The direction of the zero line is then (Fig. 12 b ) $O a \| D B$. The distance between the point $E$ and the chord $D B$ (Fig. 13 a) is $H E=0,075 r$, hence $M_{E}=1 \times 0,075 r=0,075 r$. The section $O a=0,075 r$ is drawn from the point $O$ (Fig. 13 b ) parallel to $D B$, on an arbitrarly chosen scale of forces.

If the force $P$ has the direction $C D$, the direction of the reaction at $B$ is $B C$ and the direction of the reaction at $A$ is $A C$. It appears from the triangle of forces $C b c$ (Fig. 14), in which $C b=1$, that $C c=B=0,925 r$. The arm of the moment $F D=0,29 r, E G=0,216 r$, hence the moment in $D$,

$$
\begin{aligned}
& M_{D}=0,925 \cdot 0,29 r=0,268 r \\
& M_{E}=0,925 \cdot 0,216 r=0,200 r
\end{aligned}
$$

In Fig. $12 \mathrm{~b}, O b \| C D, O b=0,268 r$. The axis of symmetry of the section $O b$ cuts in $S$ the vertical to $O a$ drawn from $O$, being the centre of a circle of the radius $O S$.

The maximum moment is to be found at $D$, when the force applied at $D$ has the direction $D$. The value of this moment is $M_{D \max }=2 \times O S=O K$.

Let us take $O b=0,2 r$ parallel to $C D$ (Fig. 13 b ). The circle passing through $O, a$ and $b$, the centre of which is to be found at the symmetrical point of intersection of the sections $O a$ and $O b$, determines the value $M_{E}$ for all directions of the force $P$ and $M_{E \max }=O K$. In Fig. 13 a is shown the direction of the force $P$, which causes the maximum moment at $E$.


Fig. 14.
Example 4. The force $P$ acts at the angle $\alpha$ to the horizontal at the middle of the tension boom of a two-hinged frame (Fig. 15 a ).

Moment at $E$ :
for

$$
\begin{gathered}
P_{h}=1, \quad M_{E}=0 \text { (symmetry) } \\
\text { for } P_{v}=1 \\
H=\frac{3}{8} \frac{l}{h} \frac{P}{2 k+3}=\frac{3 \cdot 2}{8 \cdot 4}=\frac{3}{16}
\end{gathered}
$$

since

$$
k=h: l=\frac{1}{2}
$$

$$
M_{E}=\frac{P_{v}}{2} \cdot \frac{1}{2}-H h=l\left(\frac{1}{4}-\frac{3}{16.2}\right)=\frac{5}{32} l
$$

the drawing 15 b is the graphic representation of $M_{E}$.
The moment at $B$ :
for

$$
\begin{aligned}
& P_{h}=1, \quad H=\frac{1}{2} \\
& M_{B}=-\frac{h}{2}=-\frac{l}{4}
\end{aligned}
$$

for

$$
P_{v}=1, \quad H=\frac{3}{16}, \quad M_{h}=-\frac{3}{16} h=-\frac{3}{32} l
$$

compare Fig. 15 c.

## Exceptional cases.

All cases in which the law of superposition cannot be applied are exceptions from the formulae given above. We shall consider some of them.


Fig. 15.

1. Continuous beam not anchored, on supports. The forces at the supports of such a beam must be positive. When the weight of the beam may be neglected, i.e. when the reaction caused by the dead load is very small in comparison with the reaction caused by the force $P$, the circle corresponding to the negative reactions falls out of the graphic 5 (Fig. 16).


Fig. 16.


Fig. 17.


Fig. 18.

If the reaction $R_{g}$ caused by the constant dead load of the continuous beam is greater than the greatest possible negative reaction $R_{p}$ max caused by the concentrated force $P$, the influence of the direction of this force is identical with the influence in a continuous beam anchored to both supports. Finally, if the reaction $R_{g}<R_{p \text { max }}$, only one part remains of the circle corresponding to negative reactions, viz. the part in which the distance from $O$ is smaller than $R_{g}$ (Fig. 17). The arc drawn from the centre $O$ with a radius $R_{g}$ cuts the remaining part from the negative circle.
2. A system of two rods lying in one straight line (Fig. 18) hinged together and also fixed to the supports by means of hinges. This is the limiting case of a three-hinged arch, with camber equal to $O$.

The force $P$ acts on the middle hinge in a certain direction. Its vertical component is

$$
\begin{equation*}
V=P \sin \alpha \tag{19}
\end{equation*}
$$

The vertical displacement of the middle hinge

$$
\begin{equation*}
\delta=l \sqrt[3]{\frac{V}{E F}} \tag{20}
\end{equation*}
$$

where $F=$ is the cross-sectional area of the rod and $E=$ the moduls of elasticity of the material of the rod.

Introducing (19) into (20)

$$
\begin{equation*}
\delta=l \sqrt[3]{\frac{P}{E F}} \cdot(\sin \alpha)^{\frac{1}{3}} \tag{21}
\end{equation*}
$$

The equation (21) has the general form

$$
\begin{equation*}
r=c \sin ^{n} \alpha \tag{22}
\end{equation*}
$$

besides which,

$$
\begin{gather*}
c=l \sqrt[3]{\frac{P}{E F}},  \tag{22a}\\
n=\frac{1}{3}
\end{gather*}
$$

This represents a certain curve expressed in polar coordinates $r$ and $\alpha$. The rectangular coordinates are

$$
\begin{aligned}
& y=r \sin \alpha \\
& x=r \cos \alpha
\end{aligned}
$$

Introducing (22),

$$
\begin{align*}
& y=c \sin ^{n+1} \alpha  \tag{23}\\
& x=c \sin ^{n} \alpha \cos \alpha \tag{24}
\end{align*}
$$

hence

$$
\begin{equation*}
x^{2}+y^{2}=r^{2}=c^{2} \sin ^{2 n} \alpha \tag{25}
\end{equation*}
$$

According to (23)

$$
\begin{equation*}
y^{2}=c^{2} \sin ^{2 n+2} \alpha \tag{26}
\end{equation*}
$$

Dividing (26) by (25),

$$
\frac{y^{2}}{x^{2}+y^{2}}=\sin ^{2} \alpha
$$

Raising this formula to the $(n+1)^{t h}$ power, and considering the formula (26):

$$
\left(\frac{y^{2}}{x^{2}+y^{2}}\right)^{n+1}=\frac{y^{2}}{c^{2}},
$$

it follows that

$$
\begin{equation*}
\left(x^{2}+y^{2}\right)^{n+1}=c^{2} y^{2 n} \tag{27}
\end{equation*}
$$

In our case $n=\frac{1}{3}$, and
or

$$
\left(x^{2}+y^{2}\right)^{\frac{4}{3}}=c^{2} y^{\frac{2}{3}}
$$

$$
\left(x^{2}+y^{2}\right)^{4}=c^{6} \dot{y}^{2}
$$

Introducing the dimensionless coordinates

$$
\begin{equation*}
\xi=\frac{x}{c}, \quad, \quad=\frac{y}{c}, \tag{28}
\end{equation*}
$$

it follows that

$$
\begin{equation*}
v_{i}=\left(\xi^{2}+r^{2}\right)^{2} \tag{29}
\end{equation*}
$$



Fig. 19.
In order to realize the form of the corresponding curve, $\xi_{\max }$ and a corresponding $\eta=\eta_{1}$ are to be determined. Differentiating (29) we obtain:

$$
d \eta_{i}=2\left(\xi^{2}+\iota_{i}^{2}\right)\left(2 \xi d \xi+2, d \eta_{i}\right)
$$

Dividing by $4 d \eta$ we get

$$
\frac{1}{4}=\left(\xi^{2}+r^{2}\right)\left(\xi \frac{d \xi}{d r_{1}}+r_{i}\right)
$$

Introducing $\frac{d \xi}{d \iota_{1}}=0$, we have:

$$
\begin{equation*}
\frac{1}{4}=\left(\xi_{\max }^{2}+r_{1}^{2}\right) r_{1} \tag{30}
\end{equation*}
$$

As per equation (29)

$$
\begin{equation*}
\xi^{2}+r^{2}=\sqrt{r} \tag{31}
\end{equation*}
$$

therefore

$$
\begin{gathered}
\frac{1}{4}=r_{1} \sqrt{r_{1}} \\
y_{1}^{3}=\frac{1}{16} r_{1}=0,398, \quad y_{1}=0,398 c .
\end{gathered}
$$

As per equation (31)

$$
\xi_{\max }^{2}=\sqrt{0,398}-0,398^{2}=0,63-0,158=0,472
$$

Therefore

$$
x_{\max }=c \sqrt{0,472}= \pm 0,688 c
$$

Fig. 19 shows the relation between $\delta$ and $\alpha$, for $c$ compare equation (22a).
Equation (27) will be also valid in the case of pressure of the cylinder of a movable bearing on a flat supporting plate. According to the Hertz formula, the greatest compression stress between a cylinder of diameter $r$
and length $s$, and its supporting plate, will be, with reaction $B$ :

$$
p_{\max }=0,418 \sqrt{\frac{B E}{r s}}
$$

but

$$
B=V \frac{a}{l}=P \frac{a}{l} \sin \alpha
$$

therefore

$$
p_{\max }=c \sqrt{\sin \alpha}, \text { hence } n=\frac{1}{2}
$$

if

$$
c=0,418 \sqrt{\frac{P E}{r s} \cdot \frac{a}{l}}
$$



Fig. 20.
The horizontal displacement of an antennae-tower, fixed from above with stretched ropes is another example of a system where the law of superposition cannot be applied. Here the displacement is not proportional to the horizontal component $H$, and is therefore not linearly proportional to $P \cos \alpha$, but depends on it in a more complicated way. (Compare Chmielowisc: Die Abhängigkeit der Horizontalkraft de gespannten Seiles von der Verschiebung seiner Aufhängepunkte. Bautechnik 1931, page 737.)

We have up to now considered mechanical functions being the consequences of force $P$ and having a constant direction, as, for instance, the vertical or horizontal components of the displacement or of the reaction. The reaction $R$, or the displacement, can be determined as a resultant of the
horizontal component $H$ and the vertical component $V$. These components may be obtained graphically (Fig. 20). The vertical component reaches its maximum

$$
V_{\max }=\frac{b}{l}, \text { when the force } P=1 \text { acts vertically }\left(\alpha=90^{\circ}\right)
$$

In this case the horizontal component is equal to zero. It reaches the maximum $H_{\text {max }}=1$, for $\alpha=0$. The following construction results therefore: A circle of diameter 1 is drawn tangential at $A$ to the vertical line, and a circle of diameter $\frac{b}{l}$ tangential at $A$ to $A B$. The straight line $A$ [2], parallel to $P_{2}$, cuts both circles. The chord of the smaller circle represents the vertical component of reaction $V_{2}$; the chord of the larger one the horizontal component $H_{2}$. The diagonal $A 2$ of the rectangle of base $H_{2}$ and height $V_{2}$ represents the reaction $R_{2}, P$.


Fig. 21.
The vertical reaction of the left support of a girder on two supports (Fig. 20).

$$
\begin{equation*}
V=P \sin \alpha \frac{b}{l}=P k \sin \alpha \tag{a}
\end{equation*}
$$

The vertical reaction

$$
\begin{equation*}
H=P \cos \alpha \tag{b}
\end{equation*}
$$

therefrom

$$
\begin{equation*}
R^{2}=V^{2} \dot{+} H^{2}=P^{2}\left(k^{2} \sin ^{2} \alpha+\cos ^{2} \alpha\right) \tag{c}
\end{equation*}
$$

The relation between $R$ and $\alpha$ can be determined in rectangular coordinates $X$ and $Y$, besides

$$
\begin{equation*}
X=R \cos \alpha, \quad Y=R \sin \alpha \tag{d}
\end{equation*}
$$

Eliminating $\alpha$ from equations $(c)$ and $(d)$, we obtain:

$$
R^{4}=P^{2}\left(k^{2} Y^{2}+X^{2}\right)
$$

According to equation (b)

$$
R^{2}=X^{2}+Y^{2}
$$

therefore

$$
\begin{equation*}
\left(X^{2}+Y^{2}\right)^{2}=P^{\underline{2}}\left(k^{2} Y^{2}+X^{2}\right) \tag{e}
\end{equation*}
$$

This is a curve of the 4 th degree. Equation $(e)$ remains unchanged when introducing $-X$ instead of $X$ and $-Y$ instead of $Y$, the curve has therefore a centre. Differentiating the formula $(c)$ with respect to $\alpha$ we obtain:

$$
\frac{d R^{2}}{d \alpha}=P^{2}\left(2 k^{2} \sin \alpha \cos \alpha-2 \sin \alpha \cos \alpha\right)=P^{2} \sin 2 \alpha\left(k^{2}-1\right)
$$

$R=\max (\min )$ for $\sin 2 \alpha=0$, therefore $\alpha=0$ and $\alpha=\frac{\pi}{2}$.

The curve is symmetrical with respect to the horizontal and vertical axes. It is similar to an ellipse.

The directions of the reaction $R$ and of the force $P$ are generally different; compare Fig. 21.

Equation $\alpha=\beta$ holds good only when $\alpha=0$ and $\alpha= \pm \frac{\pi}{2}$. According to (a) and (b)

$$
\tan \beta=\frac{V}{H}=K \tan \alpha
$$

If the force $P$ is acting in the middle of the girder, then

$$
\tan \alpha=2 \tan \beta=z .
$$

The deviation between the direction of the force and the direction of the reaction is

$$
\gamma=\alpha-\beta=\arctan z-\arctan \frac{z}{2}
$$

and reaches its maximum, when

$$
\frac{d \gamma}{d z}=\frac{1}{1+z^{2}}-\frac{1}{2} \cdot \frac{1}{1+\left(\frac{z}{2}\right)^{2}}=0
$$

Hence

$$
\begin{array}{rlrl}
z & =\sqrt{2}=\tan \alpha & \beta=35^{\circ} 15^{\prime} . \\
\tan \beta & =\frac{\sqrt{2}}{2}=\cot \alpha & & \alpha=54^{\circ} 45^{\prime} .
\end{array}
$$

The greatest deviation is $\gamma_{\max }=19^{\circ} 30^{\prime}$.

## Summary <br> see "Conclusions", page 38.

## Zusammenfassung.

1. In jedem System zeichnen sich für eine beliebige mechanische Größe $M$ für jeden Angriffspunkt der Kraft zwei zueinander rechtwinklig-stehende Richtungen aus, die dadurch charakterisiert sind, daB $M=0$ bezw. $M=$ max wird, wenn die Kraft in der einen, bezw. in der andern dieser Richtungen wirkt.
2. Ist eine bestimmte Größe für einen bestimmten Winkel $\alpha$ gleich Null, dann ist sie auch Null für einen Winkel $\alpha= \pm 180^{\circ}$. Für $\alpha= \pm 90^{\circ}$ dagegen erreicht die Größe ihr Maximum.
3. Für einen beliebigen Punkt des Systems und für eine beliebige mechanische Größe dieses Systems gibt es immer eine Kraftrichtung, für die die Größe gleich Null ist. Das gleiche gilt für die entgegengesetzte Richtung. Für winkelrechte Richtungen erreicht die Größe ihr pos. resp. neg. Maximum. Es ist unmöglich, daß für zwei zueinander nicht winkelrechte Richtungen die Größ en die Werte $M=0$, bezw. $M=$ max annehmen könnten.
4. Wirkt auf einen bestimmten Punkt eines Trägers oder eines beliebigen Systems eine Kraft von konstantem Werte und veränderlicher Richtung, so verursacht ${ }^{-}$sie in allen mechanischen Größen dieses Systems, die eine konstante Richtung haben, Änderungen von Null bis + max und von + max bis Null bei jeder vollen Umdrehung der Kraftrichtung.

## Résumé.

1. Dans chaque système, pour une grandeur d'ordre mécanique arbitraire et pour un point quelconque d'application de l'effort, on distingue deux directions perpendiculaires entre elles, et qui sont caractérisées par ce fait qu'elles correspondent respectivement à $M=0$ et $M=$ max, suivant que l'effort s'exerce suivant l'une ou l'autre de ces directions.
2. Si une grandeur déterminée se trouve être nulle pour un angle déterminé $\alpha$, elle sera également nulle pour tout angle $\alpha \pm 180^{\circ}$. Pour $\alpha \pm 90^{\circ}$, par contre, la grandeur considérée atteint son maximum.
3. Pour un point arbitraire du système et pour une grandeur d'ordre mécanique de ce système, il existe toujours une direction d'effort pour laquelle cette grandeur est égale à zéro. Il en est d’ailleurs de même pour la direction opposée. La grandeur atteint son maximum, positif ou négatif, pour les directions perpendiculaires. Il n'est pas possible que les grandeurs considérées passent à la fois par les valeurs $M=0$ et $M=\max$, pour deux directions qui ne seraient pas perpendiculaires, entre elles.
4. Si un effort de valeur constante et de direction variable agit en un point déterminé d'une poutre ou d'un système quelconque, cette effort a pour conséquence, pour toutes les grandeurs mécaniques du système, qui sont caractérisées par une direction déterminée, des variations entre 0 et $+\max$ et entre $+\max$ et 0 à chaque révolution complète de l'effort.

